DECODING EUROCODES 2 + 7: DESIGN OF FOUNDATIONS

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Outline of talk

April 2010: the death of British Standards? UK implementation of Eurocodes Verification of strength: limit states STR and GEO Some technical pitfalls Guidance on use of Eurocodes

APRIL 2010: THE DEATH OF BRITISH STANDARDS?



British Standards for foundations prior to April 2010



European design standards for foundations (Eurocodes)



European specifications for materials (and testing standards)

EN 206-1: Concrete EN 10025: Steel



European execution standards

EN 13670: Concrete

EN 1090: Steel



UK IMPLEMENTATION OF EUROCODES



The Eurocode family





BSI 'Published Documents' to support the Eurocodes

PD 6687: Concrete **PD 6695: Steel** PD 6688: Actions \sim

PD 6694: Geotechnical PD 6698: Earthquake resistance

'De facto' UK standards for basement design



HA BD 42: Embedded retaining walls and bridge abutments HA BD 74: Foundations CIRIA C580: Embedded retaining walls – guidance for economic design ICE: Specification for piles and embedded retaining walls IStructE: Deep basements including cut-and-cover structures Arcelor: Piling Handbook

VERIFICATION OF STRENGTH: LIMIT STATES STR AND GEO



Verification of strength

Verification of strength is expressed in Eurocode 7 by:

$$E_d \leq R_d$$

 E_d = design effect of actions

 R_d = design resistance corresponding to that effect

Applies to GEO: *"Failure or excessive deformation of the ground, in which the <u>strength</u> <u>of soil or rock is significant</u> in providing resistance' EN 1997-1 §2.4.7.1(1)P*

...and to STR:

"Internal failure or excessive deformation of the structure or structural elements ... in which the <u>strength of structural materials is</u> <u>significant</u> in providing resistance" EN 1997-1 §2.4.7.1(1)P

Application of partial factors and tolerances

Actions

$$F_d = \gamma_F F_{rep}$$

Material properties

$$X_d = \frac{X_k}{\gamma_M}$$

Geometrical parameters

$$a_d = a_{nom} \pm \Delta a$$

Effects of actions $E_d = \gamma_E E \{F_d, X_d, a_d\}$

Resistances

$$R_{d} = \frac{R\left\{F_{d}, X_{d}, a_{d}\right\}}{\gamma_{R}}$$



Verification of strength for STR (structural design)





Partial factors for GEO/STR (DA1): footings, walls, and slopes

Parameter		Symbol	bol Combination 1		on 1	Combination 2		
			A1	M1	R1	A2	M2	R1
Permanent action (G)	Unfavourable	γ_{G}	1.35			1.0		
	Favourable	(γ _{G,fav})	1.0					
Variable action (Q)	Unfavourable	γ_{Q}	1.5			1.3	-	
	Favourable	-	(0)			(0)		
Shearing resi	stance (tan φ)	γ_{ϕ}		1.0			1.25	
Effective coh	esion (c')	γ_{c}						
Undrained sh	Undrained shear strength (c _u)						1.4	
Unconfined of	compressive strength (q _u)	γ_{qu}						
Weight dens	ity (γ)	γ_{γ}	•				1.0	
Bearing resis	tance (R _v)	γ_{Rv}			1.0			1.0
Sliding resist	ance (R _h)	γ_{Rh}						
Earth resista	nce (R _e)	γ_{Re}						

SOME TECHNICAL DETAILS



$f_{cd} = \alpha_{cc} f_{ck} / \gamma_{c}$ f_{ck} = characteristic compressive strength of concrete (from cylinder tests)

 γ_c = partial factor for concrete in compression α_{cc} = factor for long term effects on compressive strength and unfavourable effects resulting from the way the load is applied

Design tensile strength of concrete is:

Design strength of concrete

Design compressive strength of concrete is:

$$f_{ctd} = \alpha_{ct} f_{ctk,0.05} / \gamma_c = 0.21 \times \alpha_{ct} f_{ck}^{2/3} / \gamma_c$$

 $f_{ctk,0.05}$ = characteristic tensile strength of concrete (5% fractile value) α_{ct} = factor for long-term effects on tensile strength, etc. [f_{ck} has units of MPa in this expression]

Design strength of reinforcing steel

Design yield strength of reinforcing steel is:

$$f_{yd} = f_{yk} / \gamma_s$$

 f_{yk} = characteristic yield strength of reinforcing steel γ_s = partial factor for reinforcing steel



Factor for long-term effects on concrete strength						
Document	Reinforced co	oncrete	Plain/lightly-reinforced conc.			
	Compression	Tension	Compression	Tension		
	α_{cc}	α_{ct}	$\alpha_{cc,pl}$	$\alpha_{ct,pl}$		
EN 1992-1-1	1.0	1.0	0.8	0.8		
UK NA (Amd 1)	0.85 flexure/axial 1.0 otherwise	1.0	0.6	0.8		

CC

Long- vs short-term strength



Justification for $\alpha_{cc} = 0.85$



EZ 1992-1-1 S S S .1.6 8 PD 6687

Oct-10

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Partial factors for reinforced concrete						
Limit states	Design situation	Reinforced concrete		Reinforced concrete		Reinforcing and pre- stressing steel
		General	Piles*	γ_{s}		
		γ_{c}	$k_{f}\gamma_{c}$			
Ultimate	Persistent	1.5	1.65	1.15		
	Transient	1.5	1.65	1.15		
	Accidental	1.2	1.32	1.0		
	Seismic	1.0†	1.0+	1.0+		
Serviceability		1.0	1.0	1.0		

*Cast-in-place piles without permanent casing; $k_f = 1.1$

⁺Recommended value for situations not explicitly covered by EN 1992

Bending resistance of singly-reinforced beam

Verification of bending resistance requires:

$$M_{Ed} \leq M_{Rd}$$

 M_{Ed} = design value of applied bending moment effect M_{Rd} = design bending resistance, given by:

$$M_{Rd} = A_s f_{yd} d \left(1 - \frac{f_{yd} A_s}{2\eta f_{cd} b d} \right)$$

 A_s = area of steel reinforcement f_{yd} = design yield strength of reinforcing steel b = width of cross-section; d = its effective depth f_{cd} = design compressive strength of concrete $\eta = 1.0$

Design chats for singly-reinforced beams



Differences between design aids

Design aids for Eurocode 2 that have appeared to date present N:M interaction diagrams in subtly different ways:

Author/Guide	Axes	Contours
TT Designer's Guide	$\frac{N_{Ed}}{h^2 f_{cd}} VS \frac{M_{Ed}}{h^3 f_{cd}}$	$\frac{A_s f_{yd}}{h^2 f_{cd}}$
Concrete Centre	$\frac{N_{Ed}}{VS} \frac{M_{Ed}}{VS}$	$A_s f_{yk}$
	$\frac{N_{Ed}}{h^2 f_{ck}} VS \frac{M_{Ed}}{h^3 f_{ck}}$	$h^2 f_{ck}$
TCC Spreadsheets	N _{Ed} (kN) vs M _{Ed} (kNm)	A _s (e.g. 4B32)

Truss model for variable strut inclination method



A - compression chord, B - struts, C - tensile chord, D - shear reinforcement

Design charts for shear resistance



Assumptions used to develop N:M interaction charts



Reinforcing bars are (rotationally) symmetrical about the pile centre

Above neutral axis, concrete and reinforcing bars provide compression force

Below neutral axis, only reinforcing bars provide tension

Shape of concrete in compression = segment of circle; must deduct area of steel in compression zone from it

Circular pile (D = 600 mm, c = 75 mm, ϕ_1 = 32 mm, ϕ_t = 8 mm)



MRd (kNm)

Feltham (2004), paper in *The Structural Engineer*



Comparing minimum reinforcement for concrete piles



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GUIDANCE ON USE OF EUROCODES



Guidance from SCI/BCSA/Corus/Concrete Centre



Designers' Guides, 'Decoding' book and blog



'Decoding' training courses from Geocentrix

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	London 23/24 November	London 12/13 January
round investigation and testing	Design of concrete foundations	Design of steel foundations
indards for ground nvestigation and testing rocode 7 Part 2 Ground nvestigation and testing inning site investigations + workshop ound characterization + workshop	Standards for structural design of concrete foundations Basis of design for concrete foundations Structural design of spread foundations + workshop Structural design of semi-gravity walls + workshop	Standards for structural design of steel foundations Basis of design for steel foundations Structural design of sheet pile walls + workshop Structural design of steel bearing piles + workshop
eotechnical reports (GDR and GIR) + workshop	Structural design of pile foundations + workshop Connections between structure	Structural design of walings + workshop Connections between structure and foundation
	ndards for ground westigation and testing boode 7 Part 2 Ground westigation and testing aning site investigations + rorkshop und characterization + rorkshop otechnical reports (GDR and	Adards for ground vestigation and testing boode 7 Part 2 Ground boode 7 Part 2 Ground vestigation and testing boode 7 Part 2 Ground boode 7 Part 2 Grou

'The Eurocode Scream'



FURTHER INFORMATION IS AVAILABLE AT

Eurocode 7 training: www.geocentrix.co.uk Eurocode 7 book: www.decodingeurocode7.com Eurocode 7 blog: www.eurocode7.com

